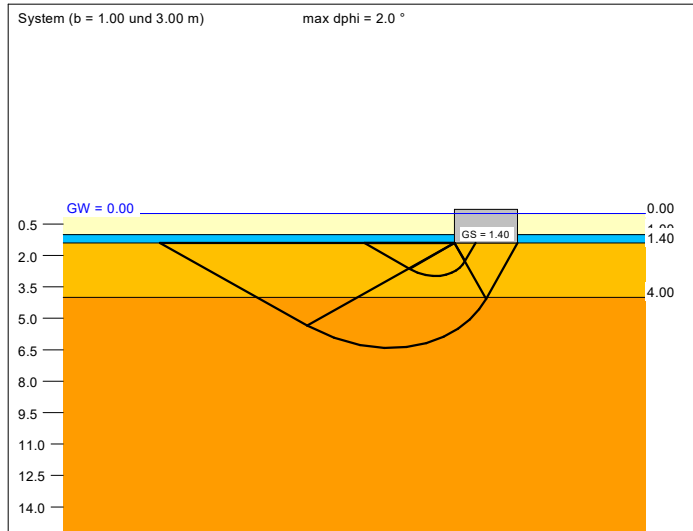
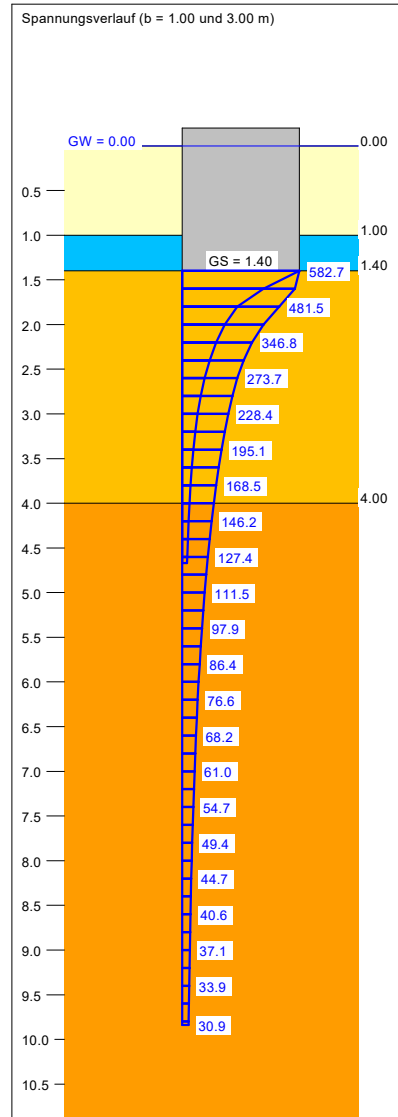


Boden	γ [kN/m³]	γ' [kN/m³]	φ [°]	c [kN/m²]	E [MN/m²]	ν [-]	Bezeichnung
	21.0	12.0	27.5	5.0	44.6	0.30	Geländeauflage
	19.0	9.0	25.0	10.0	4.7	0.40	Ton
	20.0	11.0	30.0	0.0	29.7	0.30	Sand
	20.0	11.0	32.5	0.0	44.6	0.30	Sand



a [m]	b [m]	$\sigma_{R,d}$ [kN/m²]	$R_{n,d}$ [kN]	$\sigma_{E,k}$ [kN/m²]	s [cm]	cal φ [°]	cal c [kN/m²]	γ_2 [kN/m³]	σ_0 [kN/m²]	t_g [m]	UK LS [m]
1.00	1.00	362.8	362.8	254.6	0.53	30.0	0.00	11.00	15.60	4.67	2.99
1.10	1.10	368.3	445.7	258.5	0.59	30.0	0.00	11.00	15.60	4.92	3.14
1.20	1.20	373.9	538.4	262.4	0.65	30.0	0.00	11.00	15.60	5.17	3.30
1.30	1.30	379.4	641.2	266.2	0.70	30.0	0.00	11.00	15.60	5.42	3.46
1.40	1.40	384.9	754.4	270.1	0.76	30.0	0.00	11.00	15.60	5.66	3.62
1.50	1.50	390.4	878.5	274.0	0.82	30.0	0.00	11.00	15.60	5.89	3.78
1.60	1.60	396.0	1013.7	277.9	0.87	30.0	0.00	11.00	15.60	6.13	3.94
1.70	1.70	426.5	1232.5	299.3	1.00	30.5	0.00	11.00	15.60	6.47	4.14
1.80	1.80	445.9	1444.6	312.9	1.09	30.7	0.00	11.00	15.60	6.77	4.32
1.90	1.90	460.6	1662.7	323.2	1.18	30.8	0.00	11.00	15.60	7.05	4.50
2.00	2.00	474.1	1896.2	332.7	1.27	30.9	0.00	11.00	15.60	7.32	4.67
2.10	2.10	486.6	2145.8	341.5	1.36	31.0	0.00	11.00	15.60	7.58	4.85
2.20	2.20	498.4	2412.4	349.8	1.45	31.1	0.00	11.00	15.60	7.84	5.02
2.30	2.30	509.8	2696.7	357.7	1.54	31.2	0.00	11.00	15.60	8.10	5.20
2.40	2.40	520.9	3000.2	365.5	1.63	31.2	0.00	11.00	15.60	8.35	5.37
2.50	2.50	531.7	3323.0	373.1	1.72	31.3	0.00	11.00	15.60	8.60	5.55
2.60	2.60	542.2	3665.2	380.5	1.82	31.3	0.00	11.00	15.60	8.85	5.72
2.70	2.70	552.5	4027.9	387.7	1.91	31.4	0.00	11.00	15.60	9.10	5.90
2.80	2.80	562.7	4411.7	394.9	2.01	31.4	0.00	11.00	15.60	9.35	6.07
2.90	2.90	572.8	4817.1	402.0	2.10	31.5	0.00	11.00	15.60	9.59	6.24
3.00	3.00	582.7	5244.2	408.9	2.20	31.5	0.00	11.00	15.60	9.84	6.42

$\sigma_{E,k} = \sigma_{G,k} / (\gamma_{R,v} \cdot \gamma_{G,Q}) = \sigma_{G,k} / (1.40 \cdot 1.43) = \sigma_{G,k} / 1.99$ (für Setzungen)
Verhältnis Veränderliche(Q)/Gesamtlasten(G+Q) [-] = 0.50



Berechnungsgrundlagen:
Einzelfundamente
Norm: EC 7
Grundbruchformel nach DIN 4017:2006
Teilsicherheitskonzept (EC 7)
Einzelfundament (a/b = 1.00)
 $\gamma_{R,v} = 1.40$
 $\gamma_G = 1.35$
 $\gamma_Q = 1.50$
Anteil Veränderliche Lasten = 0.500

$\gamma_{(G,Q)} = 0.500 \cdot \gamma_Q + (1 - 0.500) \cdot \gamma_G$
 $\gamma_{(G,Q)} = 1.425$
Gründungssohle = 1.40 m
Grundwasser = 0.00 m
Grenztiefe mit p = 20.0 %
Grenztiefen spannungsvariabel bestimmt
— Sohlendruck
— Setzungen

